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Hydraulical Approach in Designing of Proportional Division Structures in Traditional Irrigation Schemes

Pendekatan Hidrolik dalam Perancangan Struktur Pembagian Proporsional pada Skema Irigasi Tradisional

M. Hasan Yahya¹

Abstract

To distribute water in equal and adequate ways among the farmers, farmers have been using a proportional division structure for years. The construction of the structure is made of wood or stone with the notches in rectangular form in part of its upper side. Its performance criterion established by pertinent farmers i.e. the notch width has to be proportional to the size of its command area. From hydraulic point of view, however, the unreliability of these structures can be occurred and it was estimated that their performance criteria not only depend on the notch width but also the other hydraulic design criteria. The objectives of this research are: (i) to comprehend the dependence of performance of a proportional division structure on hydraulic design criteria, (ii) to know relationships between the discharge of structure and the hydraulic design criteria. Result of experiment indicated that the performance of a proportional division structure not only depend on notch width but also the other hydraulic design criteria in common. In general, there were the relationships between discharge of a notch and the hydraulic design criteria including notch width, thickness and surface roughness of the structure, and deviation angle of channel where the structure to be located.

Keywords: traditional Irrigation system, proportional division structure, physical model, dimension analysis

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Introduction

A traditional irrigation system, also known as farmer managed irrigation, performed with or without a financial aid of the government and fully managed by pertinent farmers. These irrigation schemes cover at least 1,036,613 ha or about 21 % of total irrigated areas in Indonesia (Ambler, 1992). The systems are facing to a number of difficult problem caused by changes in ecosystem (as consequence of deforestation), the status of water as natural resources (the priorities of water utilization established) and social economic condition in the area. One of difficult problems faced by irrigation system management of the management of is how to distribute water in equal and adequate ways among the farmers. The distribution water in unacceptable way by part of the farmers can lead to social conflict.

As an endeavor of overcoming the problem, the farmers has been using a proportional division structure for years, or may be for centuries, especially in the area with limited water supply. The performance criterion of this structure established

by the farmers i.e. the notch width has to be proportional to the size of its command area. In its operation, the proportional division structure is set at such the branching of a channel that the vicinity of structure constitute a non alignment channel (corner or concave wall). So the branches of channel form an angle with main channel in a variety of deviation angle.

Field experience indicated that the reliability of farmer's designed and operated the division structures were not always good enough. As consequence, the farmers must regularly modify the size of the notch which they established formerly. Oftentimes, it was also difficult to make a consensus among the involved farmers in the case that a farmer has a large rice area or there was a social problem which was difficult to be solved.

From hydraulic point of view, however, the unreliability of these structures can be occurred and it was estimated that their performance criteria not only depend on the notch width but also the deviation angle (Subramanya, 1983), flow properties (Chow, 1986), and surface roughness (James, 1986). Moreover, the performance of the division structure

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has never been studied such a further that was estimated the performance also vary with the other hydraulic properties and hence can deviate from the formance base on above criterion.

According to Walker (1989) design process of an irrigation structure covers matching procedure between desirable water requirement for farmers in one hand and water supply capacity in the other hand. Therefor the limits of water supply to meet optimum irrigation adjustment must be evaluated. In addition, slope and size of channel, location and size of division structure must be determined. These natures of task need certain task requirements of human system. In this research, the design process restricted to hydraulic solution only with less attention on capability of operators and users to operate system in propper way.

The objectives of this reseach are : (i) to comprehend the dependence of performance a division structure on hydraulic design criteria, (ii) to know relationships between the discharge of structure and the hydraulic design criteria including flow properties, notch width, the thickness and surface roughness of the structure, and deviation angle of channel where the structur to be located.

The usefullness of this research are: (i) to provide the knowledge in designing the proportional division structure properly by using physical model, and (ii) can be the input in evaluating, operating, and developing the existing proportional division structure.

Proportional Division Structure

A proportional division structure in a farmer managed irrigaton system has a simple form in common, made of available material in the area such as wood stave, wood stem, stone, cement, or clay. Geometry of a typical division structure “Cowal” in West Java showed in Figure 1.

In its operation, a division structure set up at the branching of a channel so that its vicinity constitute a non linier alignment channel (corner, or concave wall) (Fig.2). These branches could be formed with a variety of its deviation angle (Karnaen, *et al* and Sushila in Ambler, 1992).

The division structure could commonly be found in the area with the channel bed of a steep slope such as around the valley of a hill where hydraulic jump exist, so the downstream water level can not influence the upstream water level. Usually, the

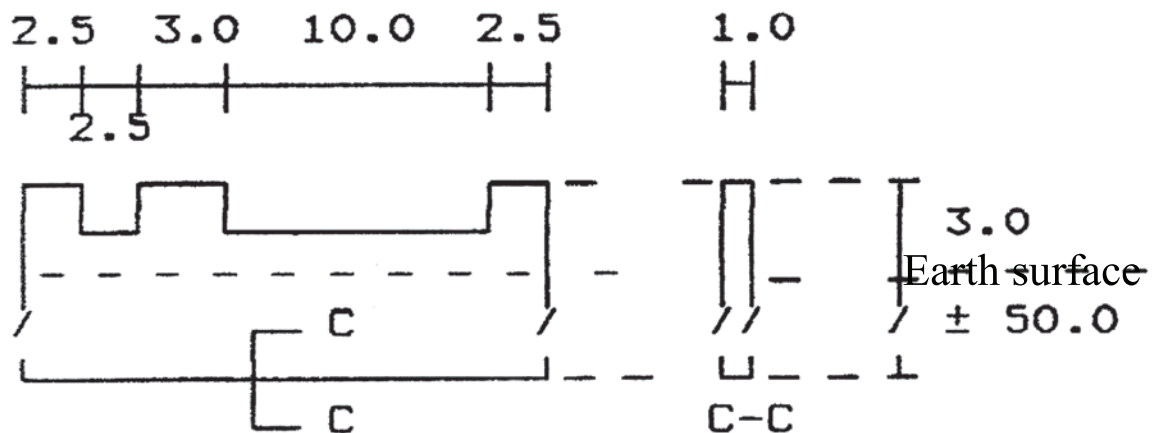


Figure 1. “ Cowal “ a typical proportional division structure in West Java (in cm): (a) longitondinal section and (b) cross section (Karnean *et al.* in Ambler, 1992)

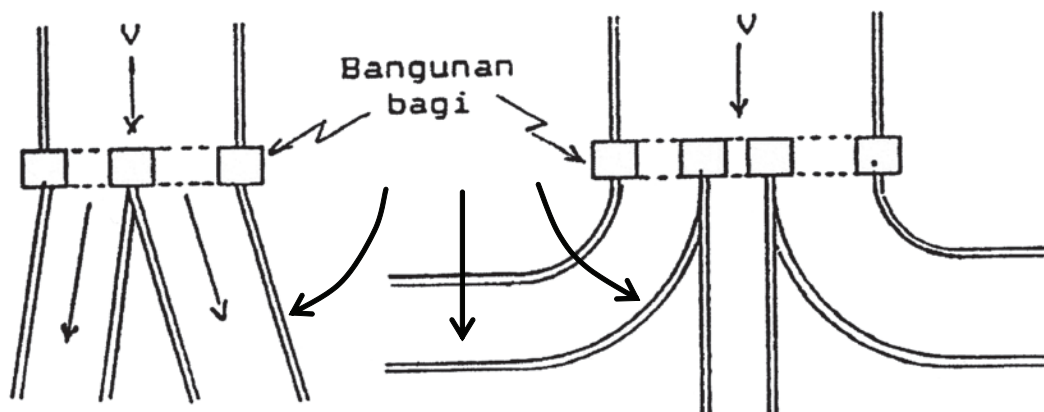


Figure 2. Setting a proportional division structure in a branching of channel: (a) at corner wall and (b) at concave wall (Karnaen, *et al* and Sushila in Ambler, 1992)

division structure is found in the area with limited water supply, conversely, in the area with sufficient water supply, this structure rarely used (Ambler, 1992).

According to James (1988) in order to function well, a division box should meet : (i) the approach channel is long and straight for at least 5 to 10 m upstream, so that flow approach the divisor in parallel path without cross currents; (ii) there is no backwater effect that would favor one side or the other; and (iii) the flow section of the structure is uniform roughness.

Mathematical Modelling

Discharge of a notch, which is constitute the function of hydraulic design criteria, can be written as follow :

$$Q_i = F(V_i, b_i, h_i, \tau_i, k_i, \theta_i, g, \rho, \mu, \sigma) \quad (8)$$

Result of dimension analysis of the above equation provides the equation in Pi term (Murphy, 1950; Munson et al., 1990) :

$$\frac{Q_i}{V_i b_i^2} = C \left(\frac{h_i}{b_i}, \frac{\tau_i}{b_i}, \frac{k_i}{b_i}, \frac{V_i^2}{gh_i}, \theta_i, \frac{\rho V_i b_i}{\mu} \right) \quad (9)$$

$$\pi_1 = C(\pi_2, \pi_3, \pi_4, \pi_5, \pi_6, \pi_7) \quad (10)$$

In open channel, to obtain dynamic similarity, viscosity force can be ignored so that π_7 can be eliminated (Simon, 1986). In addition, in case there is no backwater effect, π_6 can also be eliminated. Therefore equation 10 can be simplified as

$$\pi_1 = C(\pi_2, \pi_3, \pi_4, \pi_5) \quad (11)$$

Thereby, in accordance with Watkins (1976), several equations of design conditions can be obtained as follows :

$$h_i = n_i h_{iM} \quad (12)$$

$$\tau_i = n_i \tau_M \quad (13)$$

$$k_i = n_i k_M \quad (14)$$

$$V_i = n_i^{0.5} V_{iM} \quad (15)$$

$$l_i = n_i^{0.5} V_M \quad (16)$$

$$Q_i = n_i^{2.5} Q_{iM} \quad (17)$$

Based on equation (11), the prediction equation in Pi term written as follow (Murphy, 1950):

$$\pi_1 = \frac{\pi_{2345} \pi_{2345} \pi_{2345} \pi_{2345}}{\pi_{2345}} \quad (18)$$

If prediction discharge base on equation (18) rather distortion from theoretical formula, the right term of equation (17) must be multiplied by the prediction factor δ (Murphy, 1950), so that the equation become:

$$Q_i = \delta n_i^{2.5} Q_{iM} \quad (19)$$

Hence, total discharge prediction of a proportional division structure of i notches are

$$Q_i = \delta_1 n_1^{2.5} Q_{1M} + \delta_2 n_2^{2.5} Q_{2M} + \dots + \delta_n n_n^{2.5} Q_{nM} \quad (20)$$

The determination of prediction factor can be done as follow (in this case: $\pi_7 \neq \pi_{7M}$)

$$\pi_{7M} = a \pi_7 \quad (21)$$

$$\frac{\rho V_{iM} b_{iM}}{\mu} = \alpha \frac{\rho V_i b_i}{\mu} \quad \text{where } \alpha = \text{distortion factor.}$$

$$\alpha = n_i^{-1.5}; n_i = \frac{l_i}{l_{iM}}; \delta = \frac{f(\pi_7)}{f(\alpha \pi_7)} = \frac{C \pi_7^m}{C \alpha^m \pi_7^m} = \alpha^{-m}$$

$$\delta = (1.5^{-1.5})^{-m} \quad (22)$$

Variabel m , which constitute gradient coefficient of linear equation in log, can be determined by experiment .

$$\log(Q/V_i b_i^2) = a + m \log((\rho V_i b_i)/\mu) \quad (23)$$

Values of $\log \pi_7$ versus $\log \pi_1$ was plotted for each structure thickness as presented in Figure 3.

Expected discharge of notch i , E_i , based on design criteria established by famer can be computed as follow:

$$E_i = \frac{b_i}{b_1 + b_2 + \dots + b_n} x (Q_1 + Q_2 + \dots + Q_n) \quad (24)$$

i = number of notch 1,2, ..., n
 Q_1, Q_2, \dots, Q_n = width of notch 1, 2, ..., n
 b_1, b_2, \dots, b_n = discharge of notch 1, 2, ..., n.

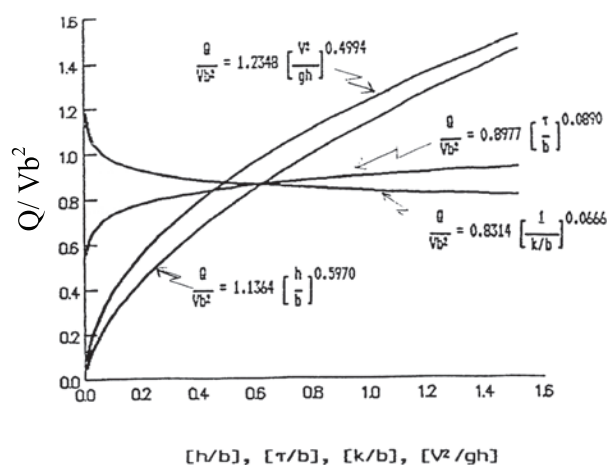


Figure 3. Diagram sketch in determination of variabel m

Table 1. Treatment composition and research variable of the model

No.	Treatment								Measurement		Research variables
	Flume	Div. struct.			Branc. chann.			Depth	Disch.		
	So	No.	k	τ (cm)	No.	θ_1	θ_2	θ_3	y (cm)	Q (l/det)	
1.	s1, s2, s3	1D	k_o	1,0	1C	20	10	30	Y ₁₂₃	Q ₁₂₃	b, θ , s
2.	s1, s2, s3	1D	k_o	1,0	2C	20	10	30			
3.	s1, s2, s3	1D	k_o	1,0	3C	20	10	30			
4.	s1, s2, s3	2D	k_o	3,0	3C	20	10	30	Y ₁₂₃	Q ₁₂₃	b, θ , s,k
5.	s1, s2, s3	3D	k_1	3,0	3C	20	10	30			
6.	s1, s2, s3	4D	k_2	3,0	3C	20	10	30			
7.	s1, s2, s3	5D	k_1	1,0	3C	20	10	30	Y ₁₂₃	Q ₁₂₃	b, θ , s, τ
8.	s1, s2, s3	3D	k_1	3,0	3C	20	10	30			
9.	s1, s2, s3	6D	k_1	6,0	3C	20	10	30			

Table 2. Treatment composition and research variable of the prototype

No.	Treatment								Measurement		Research variables
	Flume	Div. struct.			Branc. chann.			Depth	Disch.		
	So*	No.	k	τ (cm)	No.	θ_1	θ_2	θ_3	y (cm)	Q (l/det)	
1.	s1, s2, s3	1P	k_2	1,5	1C	50	30	20	Y ₁₂₃	Q ₁₂₃	b, θ , s,
2.	s1, s2, s3	1P	k_2	1,5	1C	50	30	20			
3.	s1, s2, s3	1P	k_2	1,5	1C	50	30	20			
4.	s1, s2, s3	2P	k_2	4,5	1C	50	30	20	Y ₁₂₃	Q ₁₂₃	b, θ , s,
5.	s1, s2, s3	2P	k_2	4,5	1C	50	30	20			
6.	s1, s2, s3	2P	k_2	4,5	1C	50	30	20			
7.	s1, s2, s3	5D	k_2	9,0	1C	50	30	20	Y ₁₂₃	Q ₁₂₃	b, θ , s,
8.	s1, s2, s3	3D	k_2	9,0	1C	50	30	20			
9.	s1, s2, s3	6D	k_2	9,0	1C	50	30	20			

Materials and Methods

Time and Place of The Research

This research was conducted, by experiment method, in Irrigation laboratory's Department of Agricultural Engineering, Faculty of Agriculture Technology, Gadjah Mada University, in 1995. The research was conducted under guidance of the thesis advisors. Results of research, however, never been published yet in any journal, just a thesis as part of completing study at the the partment.

Materials and Equipments

Research materials and equipments include: proportional division structures, underground reservoir, flum (main channel), branched channel, benchwork, containers, centrifugal pump, wire

screen, hook gauge, pitot tube, stopwatch, beaker glass, and pendulum balance.

Treatment Composition

Treatment composition presented in next Table 1 and Table 2.

Experimental Procedures

After all of the equipments installed (Appendix 5), water from reservoir was pumped into the container in steady state and uniform flow passing through wire screens (as energy dissipator), the porportional division structure and then pass each of the notch into branch channels. Water depth and discharge for each treatment observed at location established

Table 3. Calculated and observed discharge of proportional division structure

No.	Parameter						Discharge					
	h x10 ² (dm)	τ (dm)	k x10 ² (dm)	V for notch width:			Q _c for notch width:			Q _o for notch width:		
				b1 v x10 (dm/s)	b2 v x10 (dm/s)	b3 v x10 (dm/s)	b1 (l/s)	b2 (l/s)	b3 (l/s)	b1 (l/s)	b2 (l/s)	b3 (l/s)
1.	25.25	0.1	0.20	3.60	3.81	3.56	0.31	0.51	0.61	0.34	0.48	0.56
2.	25.40	0.1	0.20	4.83	5.00	5.00	0.55	0.88	1.20	0.58	0.80	1.00
3.	34.40	0.1	0.55	3.44	3.79	3.61	0.27	0.49	0.60	0.32	0.47	0.56
4.	39.90	0.3	0.55	3.52	3.50	3.54	0.32	0.46	0.65	0.37	0.49	0.62
5.	24.03	0.3	0.55	3.73	3.60	3.47	0.34	0.47	0.59	0.39	0.49	0.59
6.	33.90	0.3	0.90	3.33	3.39	3.55	0.27	0.41	0.62	0.31	0.42	0.55
7.	37.00	0.6	0.55	3.33	3.05	3.31	0.30	0.37	0.60	0.32	0.39	0.53
8.	42.15	0.6	0.55	3.04	2.79	3.12	0.25	0.32	0.54	0.31	0.38	0.53
9.	18.77	0.6	0.90	3.67	3.50	3.53	0.31	0.42	0.58	0.33	0.42	0.53

Results and Discussion

Flow Properties, Channel Deviation Angle, and Surface Roughness

Results of statistical analysis indicated that the performance criteria of proportional division structure is not affected significantly by either flow properties (subcritical or supercritical flow) or the deviation angle up to 50 degrees. There were the reability of structure because of no backwater effect, no cross current in upstream of structure. In accordance with suggestion by James (1988), if all condition above can be meet, then the performance of division structure can't be affected significantly by both flow properties and deviation angle (up to 50 degrees).

The division structure with surface roughness of grade k1 ≈ 0.00055 m and k2 ≈ 0.00090 m belong to criterion of hydrodynamically transition. It means the quantity of water reduced by a solid surface depend on Reynold number as well as relative roughness (Garde and Mirajgaoker, 1983). Base on the experiment indicated that the quantity of reduced water of a notch tend to be increased with decreased its notch width and conversely. It seems that the surface roughness can decreas of water to unproportional of notch width, hence for some extent can affected performance criteria of a proportional division structure.

Relationships Between Discharge and Hydraulic Design Parameters

Results of data analysis by using equation (18), it was obtained relationships between discharge of a proportional division structure and hydraulic

design criteria in the form of following mathematical equation:

$$Q = 1.1445 \frac{1}{k^{0.0666}} \left(\frac{h^{0.9638} \tau^{0.1437}}{b} \right)^{0.6194} \left(\frac{V^2}{gh} \right)^{0.4994} (vb^2) \quad (25)$$

From the equation indicated that the discharge not only depend on notch width (b) but also the others including roughness height (k), head over the crest (h), crest thickness (τ), and flow velocity (V). Results of computed discharge by using equation (25) and of observed discharge was presented in Table 3. It showed that the observed discharge can be approached closely by using the equation. Sensitivity and interrelation among them can be showed in Figure 4.

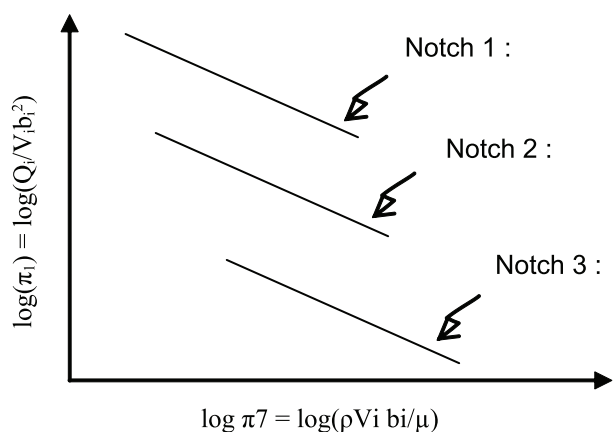


Figure 4. Sensitivity of hydraulic design parameters on performance of proportional division structure

Conclusions and Recommendations

Conclusion

1. There were the dependence of performance of the division structure on other hydraulic design criteria (beside notch width) in common. However in the case there were no backwater effect and cross current in upstream vicinity of structure, the dependence on the other hydraulic design criteria was not significantly different.
2. There were relationships between discharge of a structure (the reflection of its performance) and hydraulics design criteria which can be showed in mathematical equation.

Recommendation

To study the performance of proportional division structure so further, the research can be continued in the case there were a backwater effect and cross current in upstream of a proportional division structure.

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